

Quantum Environmental Group Barrier Wall Presentation REMTECH 2005 Banff, Alberta



Quantum Environmental Group

DIVISIONS PARTNERSHIPS • Quantum Hazmat Inc. • Envirogreen Technologies • Quantum Facilities Inc. • Windmill Developments Ltd.

•Quantum Emergency Response Inc. ALLIANCES

Burrard Clean Operations



Quantum Environmental Group

Barrier Wall Presentation

Agenda

- Clay / Clay Bentomat Barrier Walls
 Soil Bentonite Admixture Barrier Walls
 Waterloo Barrier Walls
 Bentonite Slurry Barrier Walls
 Permeable Reactive Barrier / Jet Grout Barrier Walls
 Slurry Slot Excavation / Low Strength Concrete Barrier Walls
- Caisson Walls





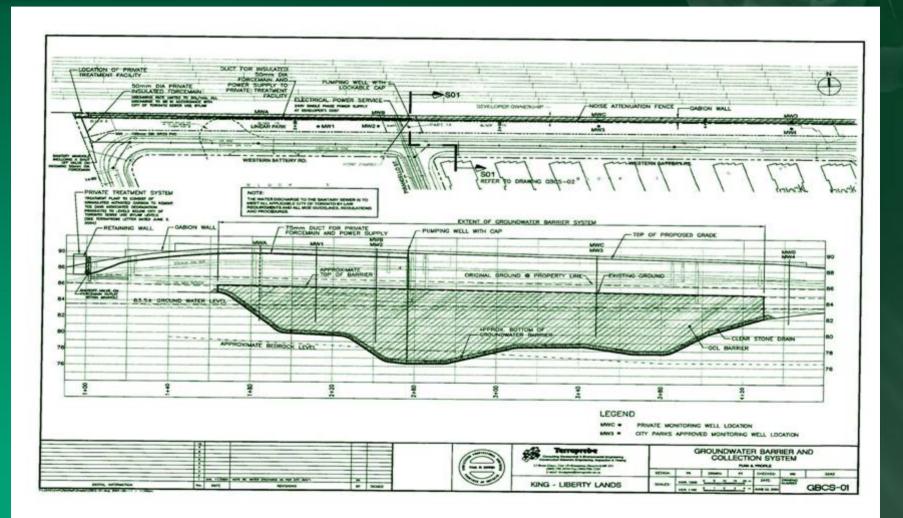
The Challenge

•Install 270lm vertical impermeable barrier at P/L, at depths from 3-10 mbg in a safe manner that minimized geotechnical and employee risk;

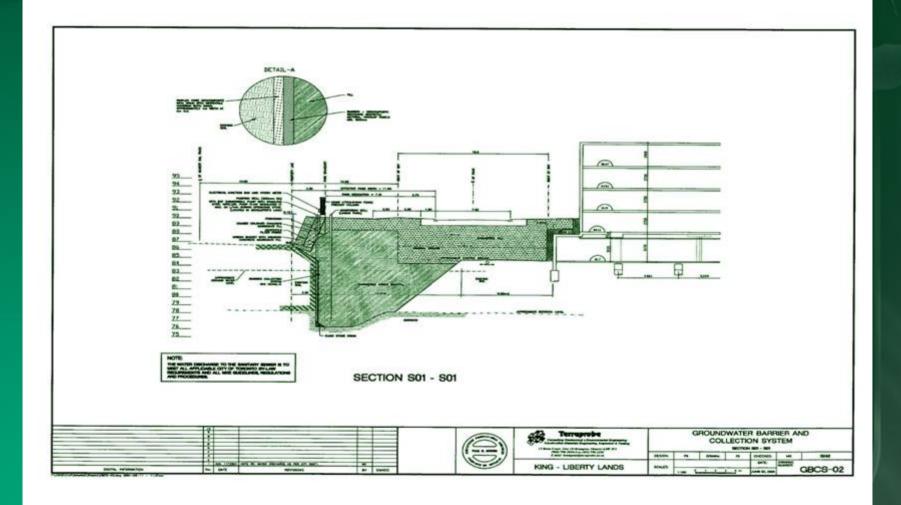
•Design permeability had to be <10E-8 m/s;

 Install upgradient drainage and dewatering system













Soil Nailing

 2m long 3/8" rebar nailed onto vertical excavation face on 2m spacing horizontal and vertical

 Hang 4" steel mesh on protruding rebar

•Tie mesh to rebar and trim protruding rebar





Install French Drain and Wick Drain

•0.5 m wedge of 19mm crush placed in base

 Wick drain material hung from lifeline and draped down vertical face

 Base elevation of barrier graded to a low collection point to reduce upgradient pore pressure





Hang Bentomat Liner

 Bentomat Liner composed of 3/8" thick bentonite stitched between 2 woven geofabrics

Cut liner at depth + 1 m

•Clamped to 4.5 m long 2x6 and hung from material lifeline by excavator

Liner covers French Drain and
0.6 m overlap on adjacent liners

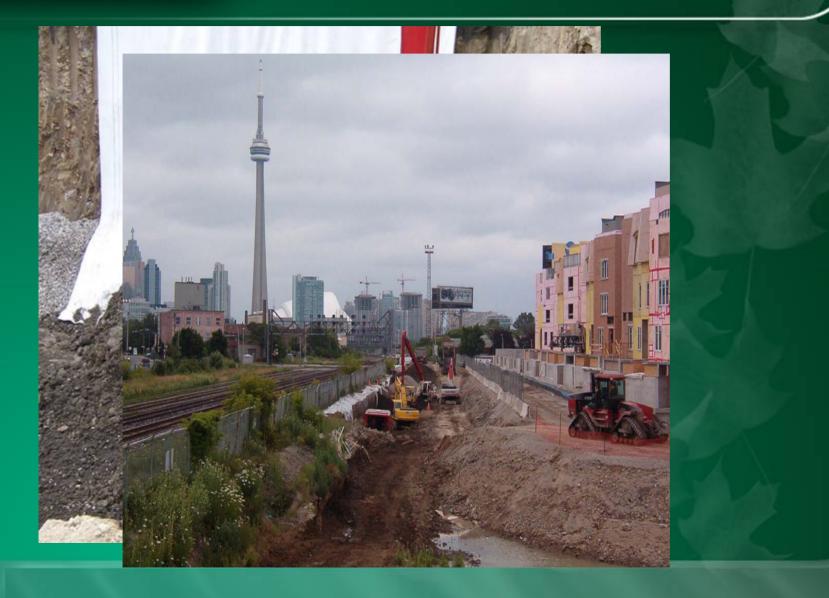
 Backfilled and compacted with hoepack at base

 Backfilled and compacted to grade using sheeps foot roller











Bentomat Liner Cut-off Wall Advantages

- Allows for variable depth profiles
- Can be manufactured to meet a design permeability up to 10E-12 m/s
- Relatively inexpensive barrier material, no field welding required as overlap and hydration form continuous barrier
- Simple to install on slopes
- Can be installed without specialized equipment





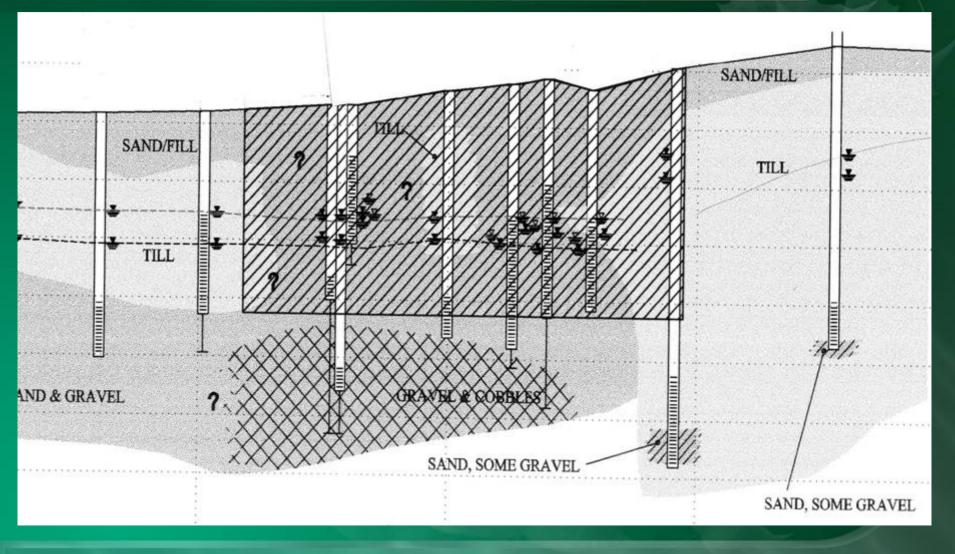
The Challenge

•Design/install an impermeable barrier to 8.5 mbg adjacent to foreshore in very dense till

•Key barrier into lowpermeability sediments to prevent short circuiting

• Design/install 13 LNAPL skimmers package in active railyard to collect LNAPL







Solution

 Dry excavation of an 8.5 m deep, 1.5 m wide trench backfilled with lightly-compacted sand

 Design, testing and placement of an engineered, lowpermeability, soil-bentonite admixture to key barrier into till sediments

 Installation Waterloo Barrier sheet pile wall, with joints flushed and grouted

Advancement of LNAPL recovery wells

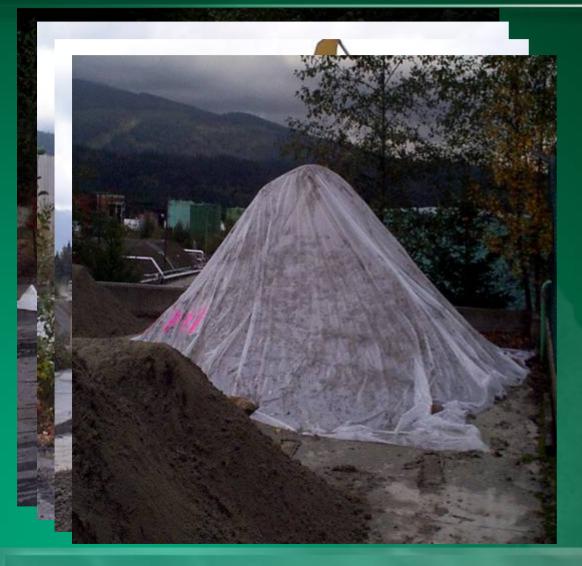
Design, fabrication, installation and commissioning of 13
 LNAPL skimmer unit complete with reservoirs and controls



Soil Bentonite Admixture Design and Testing

Silty sand (fines/silt content >20%)
Varying amounts of powdered bentonite
Compactive effort – 90 to 100% SPD
Moisture content ~10 to 12%
Constant Head permeability testing
K values between 10E-8 to 10E-12 m/s





Preparation of Soil-Bentonite Admixture

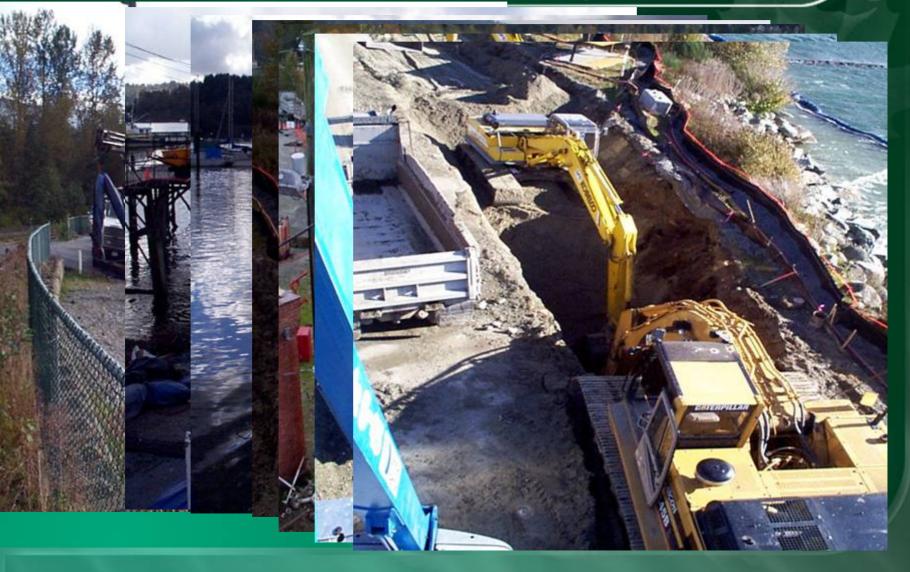
 Sieve/moisture analysis of sand stockpile

•Weighing and dry mixing of bentonite and sand

•Sieve analysis of several samples from each stockpile to confirm proper mixing and correct material quantities

 Covering of stockpiles to prevent hydration









Admixture Placement

•5 lm, 8.5 m deep at each end of trench

 Excavation to be kept dry to meet 95% SPD

 Method spec. based on lift thickness (1 m) and compactive effort (sec/m2) field tested through densometer readings

 On-site geotechnical engineer performed QA/QC penetration testing from man bucket





Erect Driving Rack

 Maintains wall alignment and plumb

 Provides safe work area for sheet handler/threader

 Sufficiently stable soils req'd to support rack, particularly during wind events





Threading Sheets

•First sheet advanced to ~50% depth

 Entire rack filled with treaded sheets advanced slightly into formation

 Once entire rack filled, sheet driving begins across entire rack

 Vibratory hammer used to advance sheets





Sheet Driving

•Rack used to drive to ~70% penetration, freely driven for remainder

 Rack re-located for next run

 Plumb checked on each sheet while driving +/–
 1% off-plumb tolerance





Joint Jetting and Grouting

•Joint cleaned of soil/rock by advancing high pressure water hose down to bottom of joint

•Joint pumped full of low-permeability grout mixture and allowed to harden

 Provides permeability of 10E-11 m/s





In-situ System Installation

 Advance 100mm diameter recovery wells

•Design, fabricate, install 13 LNAPL skimmer packages

 Install underground electrical to each skimmer

•Skimmers inc. reservoirs, secondary containment and all required electrical controls

 Commission system and prepare operations manual



Engineered Fill Cut-off Walls - Advantages

Allows for variable depth profiles

Can be engineered for various design permeabilities (10E-7 to 10E-12 m/s)

Sand gradation (> fines content lowers permeability; >20%)

Bentonite content

Compactive effort (90 to 100 SPD)

Wall thickness can be designed to meet required flow characteristics

Allows for post-installation testing of barrier material

 Engineered material is easy to handle and requires no specialized equipment to mix and install.



Waterloo Barrier - Advantages

- Allows for very deep barrier wall (up to 20m)
- Tight continuous interlock filled with grout provides barrier integrity
- Can be driven with conventional pile driving equipment
- Can be removed and reused
- Certified for 1x10E-11 m/s





The Challenge

Install a 125 m long
 impermeable barrier to 8.5
 mbg to mitigate potential
 migration of impacts into
 Quesnel River

•Design permeability to be 1x10E-8 m/s

•Barrier to be within 0.6m of an on-site structure

 Barrier located within 3m of riprap bank / river



Solution

•Work with consultants to design and subsequently install an 8.5 m deep, 1.5 m wide, 125 m long bentonite slurry wall, backfilled with an engineered, low-permeability soil admixture

 Design and laboratory testing of an engineered, lowpermeability, soil-bentonite admixture (trench backfill) to confirm K values

- Design trench slurry mixing and handling plan
- Design of soil admixture (trench backfill) mixing, handling and QA/QC plan
- Design of trench excavation and backfilling plan
- Preparation of Health and Safety Plan



Slurry Trench Excavation – Basic Principles

•Trench wall stability is maintained by excess head of bentonite slurry in the trench (slurry level must be continually maintained above existing gw level)

•Excess head maintained by minimizing loss of slurry to formation by:

Reduced K at trench interface as bentonite fills voids in formation

 Monitoring and maintaining viscosity of bentonite slurry in the trench (temp., % bentonite, % sand)

Introducing make up slurry

 Bentonite must be completed hydrated in slurry (mixing procedure, pH, temperature, cat ion conc. dependant) to maintain viscosity and remain available to fill voids at interface

 Hydraulic conductivity of formation reduced as a result of the introduction of bentonite





Field Preparation of Bentonite Slurry

 Water supply conditioned with soda ash to raise pH

•Dry powdered bentonite mixed with water using shear type pumps (mud mixers)

•Slurry re-circulated in baffled mixing tanks for full hydration

•Tested to meet a minimum Marsh cone standard of 38 seconds (viscosity test)

 Pumped to a holding basin and re-circulated in basin





Field Preparation of Soil-Bentonite Admixture (Trench Backfill)

•Weighing and dry mixing of 50% of the required bentonite with sand

 Addition/mixing of the remaining required bentonite in dissolved format (hydration)

 Addition of water to obtain 5"-6" slump test

•Slump test dictates selfplacement characteristics of trench backfill





Trench Excavation and Backfilling

1 m deep bench excavated
– control slurry and
spoil/attain design depth

 2 excavators working simultaneously excavating and backfilling

•Slurry maintained at ~1mbg (gw @ 3.5mbg)

•~15 m of trench open at once

•Trench backfill placed (3H:1V) and self compacting

 Continual depth sounding of trench to confirm depth and backfill location



Bentonite Cut-off Walls - Advantages

- Allows for variable (unknown) depth profiles and direction changes
- Can attain significant depths (limited by machine size)
- Ideal for loose formations with cobbles/gravels (precludes driven barriers)
- Trench backfill can be engineered for various design permeabilities (10E-7 to 10E-10 m/s): Sand gradation and bentonite content
- •Wall thickness can be designed to meet required flow characteristics, FOS
- Allows for post-installation testing of barrier material
- Engineered trench backfill material is relatively easy to handle and requires limited specialized equipment to mix and install
- Relatively cost-effective option

 Abundance of technical literature – level of confidence for owners/consultants



Confidential Site, BC - Design/Build – Permeable Reactive Barrier



The Challenge

- Remove source of special waste heavy metals contamination through excavation.
- Construct a special waste storage facility to accept special waste metal contaminated soils excavated from the site
- Install an in-ground passive treatment wall to treat metal contaminated water generated on the site prior to discharge to the ocean.



Confidential Site, BC - Design/Build – Permeable Reactive Barrier

Solution

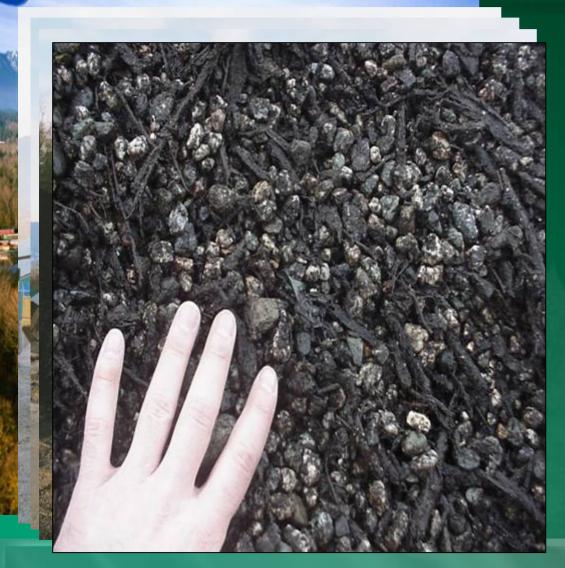
- Install a 750lm, 18m deep, 2m wide PRB to treat contaminated groundwater prior to discharge to the ocean
 - Design guar gum slurry, mixing plant, QA/QC plan
 - Mix proprietary trench media, design QA/QC plan
 - Excavate/backfill trench simultaneously monitoring depth and backfill profile
 - Monitor trench slurry QA/QC to ensure trench stability
 - Clay cap on trench
- Jet grouting in area of GVRD water mains



Confidential Site, BC - Design/Build – Permeable Reactive Barrier







Filter Media Mixing & Testing

•Media comprised 85% pea gravel, 14% compost, 1% lime

 Numerous samples from each batch mixture were tested by owner

Composition

•K (preferential pathway)

 Mixed using a loader with scale bucket and excavator

 Thorough mixing required to ensure no"dead spots' in wall





Guar Gum Slurry Mixing/ Testing

 Guar gum (a food thickener) used as i degrades over time - PRB

 Guar gum slurry mixed using oil-field mud plant (shear pump) and stored in a 20000 USG tank

Plant comprised 3 – 20000 USG tanks

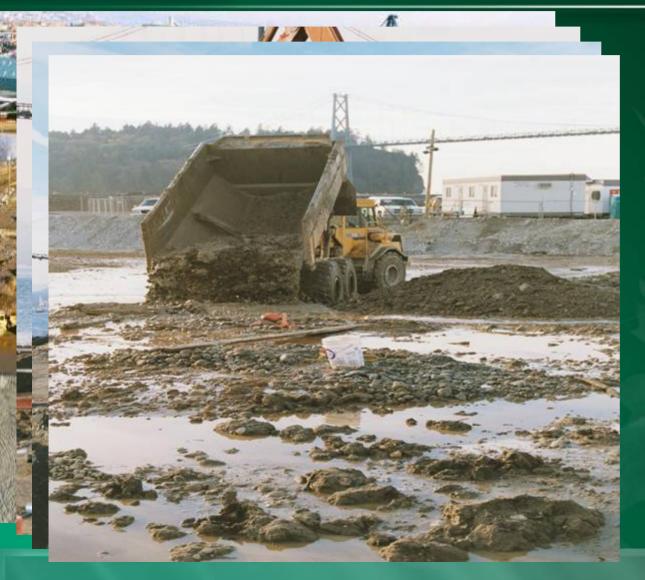
Slurry breaks down in 48 hours at 180

•Finished and trench slurry tested for pH, viscosity and bacterial count

Make up slurry always required

3m Factor of Safety in slurry trench





Trench Excavation and Backfilling

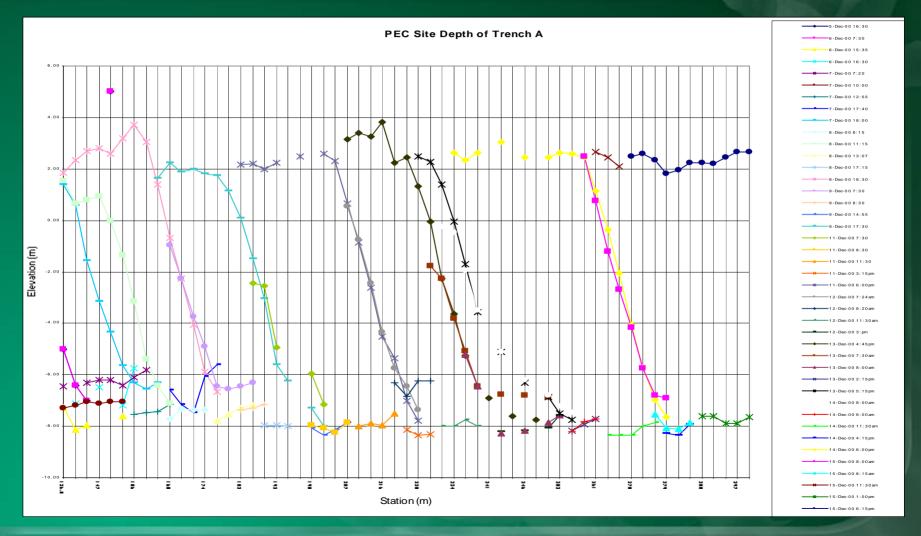
•200T excavator and 125T crane excavating/backfilling simultaneously, 50lm of trench open

•Removal of 20,000m3 of spoil, placement of 18,000m3 of filter media and clay cap

•Real time surveying and monitoring trench depth and backfill profile

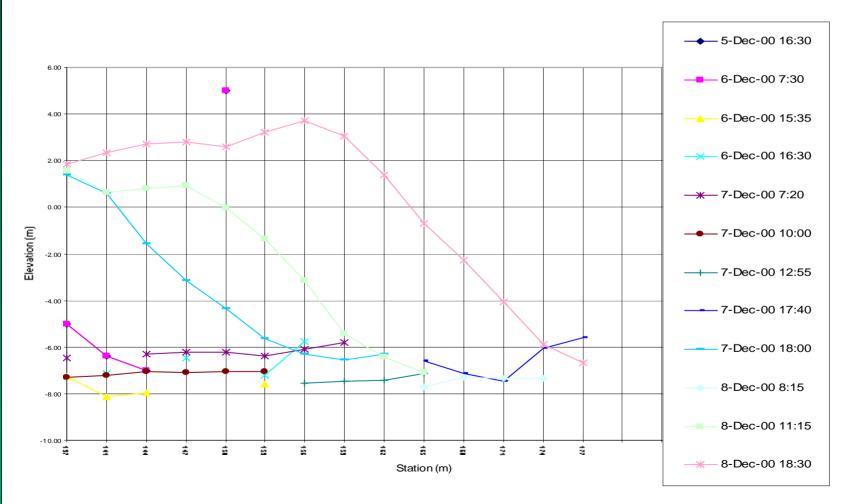
•Sheet pile restraint designed for emergency stoppages



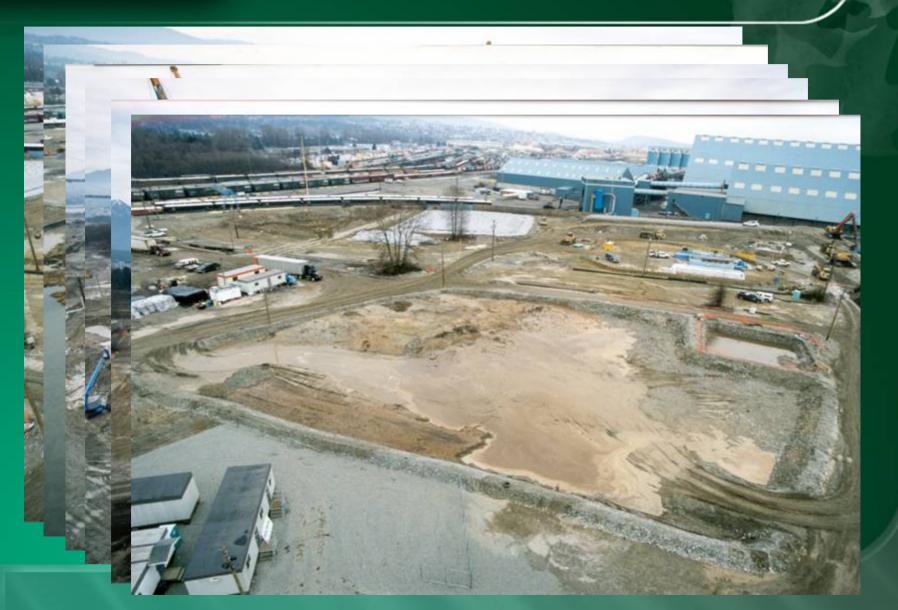




PEC Site Depth of Trench A









Gar Gum Slurry Permeable Reactive Barriers - Advantages Allows for variable (unknown) depth profiles and direction changes Can attain significant depths (limited by machine size) Ideal for loose formations with cobbles/gravels (precludes driven barriers) Guar gum slurry breaks down (digested by bacteria) over time Does not alter regional hydrogeology Minimal / no deleterious effect on fish/wildlife Trench design life can be engineered for varying duration (thickness) On-going pump and treat requirement eliminated Can be used different contaminants (media specific) Ideal for very large sites





The Challenge

•Excavate entire site to 11 m below grade – no anchors allowed on City property;

•Shoring mechanism must prevent loss of soil behind shoring wall – compromise off-site utilities

Install impermeable barrier on 3 property lines from 6 to 11m below grade;

Hydraulic conductivity to be
 < 10E-8m/s



Solution

 Installation of H-pile to 11mbg and lagging (inc. rakers) to 7mbg to shore excavation wall on N and E P/L

 Slurry slots excavations backfilled with low strength concrete from 7 to 11mbg on N and E P/Ls to remove impacted soil and provide hydraulic barrier

 Installation of caisson wall on W P/L to 11m below grade to support adjacent parade and provide hydraulic barrier

 Slot excavation of impacts at depth along caisson wall, backfilled with low strength concrete

Wet excavation of entire central portion of site





H-Pile and Lagging in North and East Portion of Site

•760 mm caissons open drilled to 11m, filled with H-pile and low strength concrete (4MPa)

•Waler installed at grade and excavation between H-Piles completed with timber lagging installed to 7 mbg

•Once at 4 mbg, rakers welded and set in 30 MPa concrete footings excavated at 7-9 mbg

•Bulk excavation completed to 7 mbg in north and east portion of site, with waler attached at 6.5mbg.









Bentonite Slurry/Concrete Wall

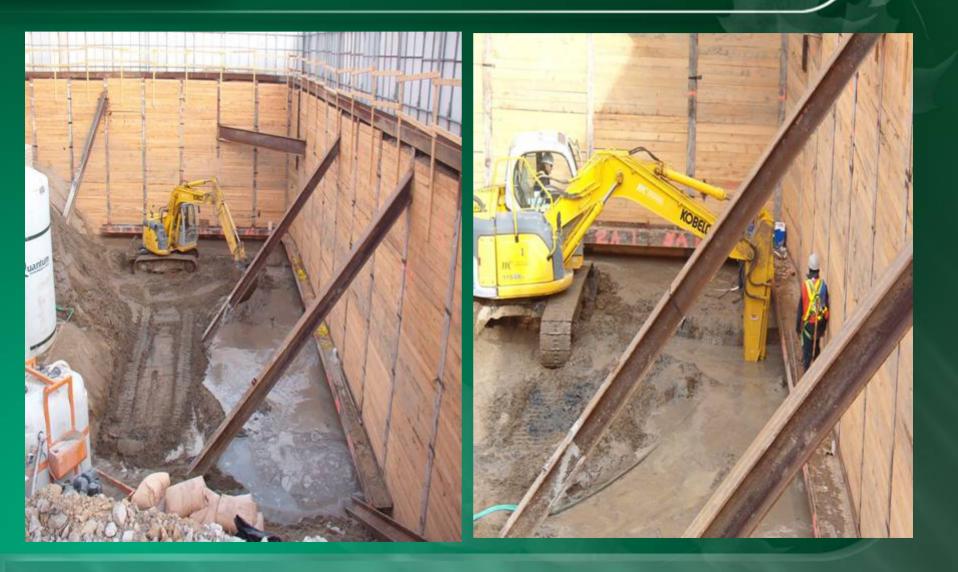
•Slots excavated to 11mbg using bentonite slurry to shore excavation wall

 Slot backfilled with low strength concrete to provide impermeable barrier

•Alternating slot technique used with over excavation into adjacent slot to ensure continuous impermeable wall

 Bentonite slurry mixed using shear pump to meet 45 second Marsh Cone viscosity test







Bentonite Slurry/Concrete Cut-off Walls – Advantages

- Allows for variable depth profiles
- Allows for installation in unconsolidated material
- Can be engineered for various design permeabilities (10E-8 to 10E-12m/s)
 - Concrete additives (bentonite) can lower permeability
- Wall thickness can be designed to meet required flow characteristics
- Low strength concrete provides temporary structural excavation support and re-excavateble.



Caisson Wall Barrier Installation

•From 3mbg bench, 13, 915mm dia. open boreholes were drilled on a specified spacing to ~11mbg and filled with an H-Pile and LS concrete to 3 mbg; H-Piles were used in select caissons to construct a shoring support wall

 Subsequently, 3, 864mm dia. 'filler' boreholes were advanced between each of the 13 previously drilled, reinforced and filled boreholes

•'Filler' boreholes were drilled with a minimum 0.2m overlap into the adjacent caisson and filled with LS concrete; 'filler' borehole 2 was drilled first, followed by 'filler' boreholes 1 and 3

•A waler was installed at 3.5 mbg (top of caisson wall) to further stabilize the cantilevered caisson wall

Slot excavations were conducted to ~11mbg, with slots backfilled with LS concrete.











Concrete Caisson Barrier Walls -Advantages

Solid structure, cantilevered excavation wall support system

•can be advanced right at property line

•no off-site encroachment or agreements necessary

•no rakers required (simplifies backfilling)

Shoring excavation support and impermeable barrier installed simultaneously

Low strength concrete can be easily excavated for site servicing (vs. pulling H-piles and lagging)